

DANVAS DEVELOPMENTS

13 ALEXANDRINA ROAD,
MOUNT BARKER
S.A. 5251

FOR PLANNING ASSESSMENT



DRIVEWAY ELEVATION LOOKING NORTH



MOUNT BARKER
DISTRICT COUNCIL

Planning Consent

AMENDMENTS			
Rev	Date	Description	Drawn
A	08.05.25	Concept Design	C.I.
B	22.05.25	Planning Issue	C.I.
C	29.07.25	Council RFI	C.I.

DANVAS

DEVELOPMENTS PTY. LTD.



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SUITE 1 / 159 PORT ROAD
HINDMARSH S.A.
T: 8338 2211 F: 8338 2188

PROJECT
PROPOSED RESIDENTIAL
DEVELOPMENT
AT:
13 ALEXANDRINA ROAD,
MOUNT BARKER

CLIENT
DANVAS DEVELOPMENTS PTY. LTD.

DRAWN
CI
DATE
MAY 2025

SCALE
AS SHOWN
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PROJECT No.
25.2025
SHEET No.
PA 01 OF 07

CONTRACTORS ARE TO VERIFY ALL DIMENSIONS
AND LEVELS ON SITE BEFORE COMMENCING ANY
WORK OR MAKING SHOP DRAWINGS. FIGURED
DIMENSIONS SHALL TAKE PREFERENCE OVER
SCALED DIMENSIONS AND ANY DISCREPANCY SHALL
BE REPORTED TO THE DESIGNERS IMMEDIATELY.



FRONT ELEVATION TO STREET



DRIVEWAY ELEVATION LOOKING SOUTH



MOUNT BARKER
DISTRICT COUNCIL

Planning Consent

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PROJECT
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CONSTRUCTION MATERIALS:

- POWER PANEL CONSTRUCTION TO UPPER AND LOWER;
- FINISHES AS SHOWN
- COLORBOND ROOF ON 2° PITCH WHERE SHOWN
- ALUMINIUM WINDOWS AND DOORS
- CONCRETE SLAB
- SELECTED PAVING TO AROUND DWELLING AND DRIVEWAY



LANDSCAPING:



1
Common Name: Boumda Beauty Wax Flower
Plant Height: 1
Plant Spread: 1



2
Common Name: James Stirling Pittosporum
Plant Height: 3
Plant Spread: 1.5



3
Common Name: Narrow Leaf Chalk Sticks
Plant Height: 0.4
Plant Spread: 1.2



4
Common Name: Orange Berry
Plant Height: 4
Plant Spread: 2

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SCALE
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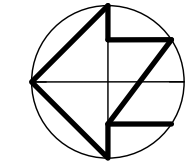
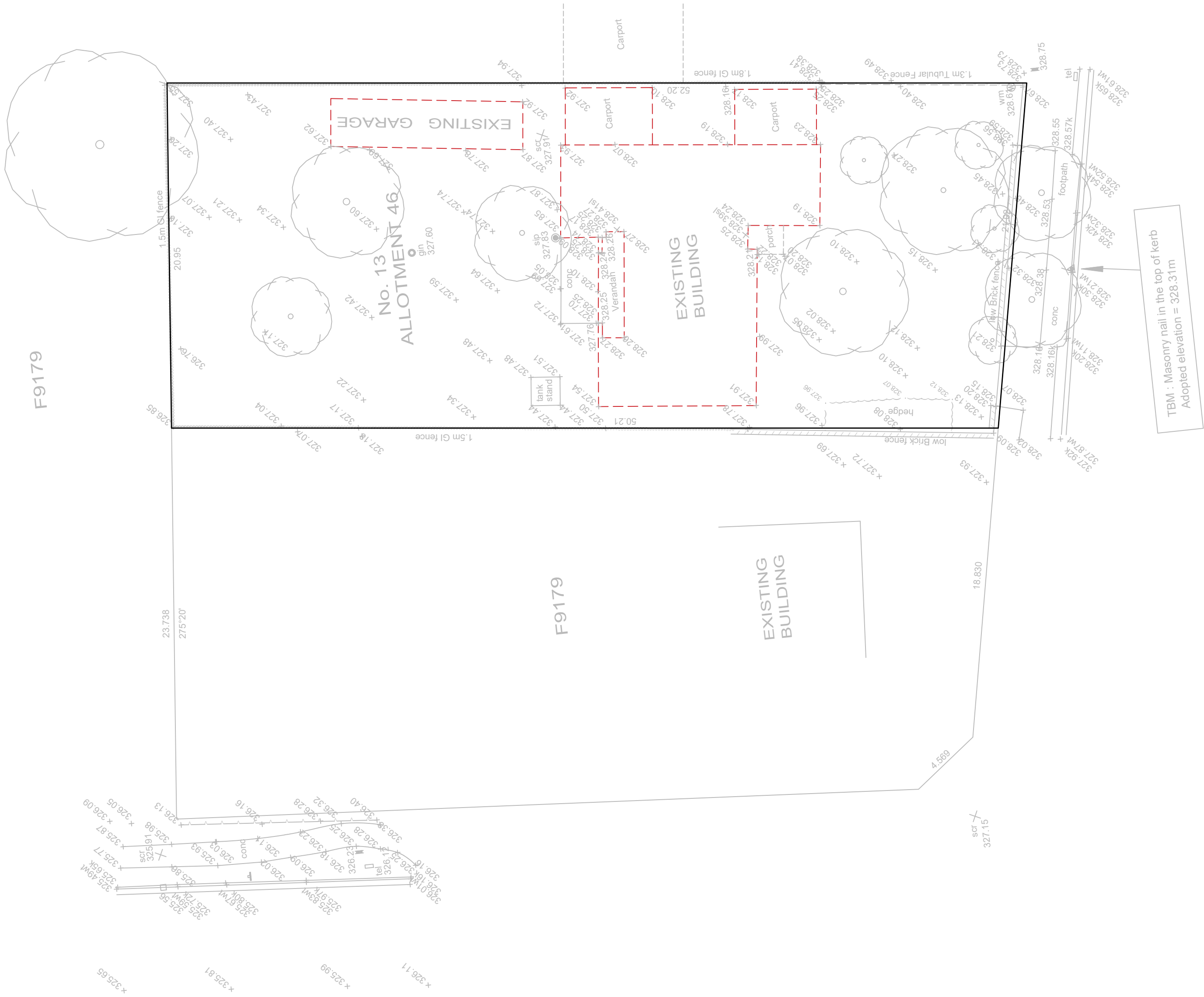
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D1 AREA :		D2-8 AREA :		D9 AREA :		SUMMARY
LOWER LIVING	47.10	LOWER LIVING	36.38	LOWER LIVING	47.79	SITE 1072.68 SQ.M.
UPPER LIVING	59.68	UPPER LIVING	58.69	UPPER LIVING	59.26	SITE COVER 48.94 %
GARAGE	19.50	GARAGE	19.50	GARAGE	19.41	SOFT L.SACPE 18.02 %
BALCONY	12.02	BALCONY	11.87	BALCONY	12.02	
PORCH	1.90	PORCH	1.82	PORCH	1.90	
TOTAL	140.20 SQ.M.	TOTAL	128.26 SQ.M.	TOTAL	140.38 SQ.M.	



SITE PLAN

SCALE 1:200

AMENDMENTS			
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C	29.07.25	Council RFI	C.I.

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PROJECT
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MAY. 2025

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GROUND FLOOR / SITE PLAN
SCALE 1:100 @ A2

D1 AREA :	D2-8 AREA :	D9 AREA :	SUMMARY
LOWER LIVING 47.10	LOWER LIVING 36.38	LOWER LIVING 47.79	SITE 1072.68 SQ.M.
UPPER LIVING 59.68	UPPER LIVING 58.69	UPPER LIVING 59.26	SITE COVER 48.94 %
GARAGE 19.50	GARAGE 19.50	GARAGE 19.41	SOFT L.SACPE 18.02 %
BALCONY 12.02	BALCONY 11.87	BALCONY 12.02	
PORCH 1.90	PORCH 1.82	PORCH 1.90	
TOTAL 140.20 SQ.M.	TOTAL 128.26 SQ.M.	TOTAL 140.38 SQ.M.	

AMENDMENTS			
Rev	Date	Description	Drawn
A	08.05.25	Concept Design	C.I.
B	22.05.25	Planning Issue	C.I.
C	29.07.25	Council RFI	C.I.

DANVAS

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PROJECT
PROPOSED RESIDENTIAL
DEVELOPMENT
AT:
13 ALEXANDRINA ROAD,
MOUNT BARKER

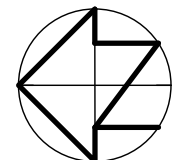
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DRAWN
CI
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MAY, 2025

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PROJECT NO:
25.2025
SHEET NO:
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UPPER FLOOR PLAN
SCALE 1:100 @ A2

D1 AREA :		D2-8 AREA :		D9 AREA :		SUMMARY
LOWER LIVING	47.10	LOWER LIVING	36.38	LOWER LIVING	47.79	
UPPER LIVING	59.68	UPPER LIVING	58.69	UPPER LIVING	59.26	
GARAGE	19.50	GARAGE	19.50	GARAGE	19.41	
BALCONY	12.02	BALCONY	11.87	BALCONY	12.02	
PORCH	1.90	PORCH	1.82	PORCH	1.90	
TOTAL	140.20 SQ.M.	TOTAL	128.26 SQ.M.	TOTAL	140.38 SQ.M.	SITE 1072.68 SQ.M.
						SITE COVER 48.94 %
						SOFT L.SACPE 18.02 %

AMENDMENTS			
Rev	Date	Description	Drawn
A	08.05.25	Concept Design	C.I.
B	22.05.25	Planning Issue	C.I.
C	29.07.25	Council RFI	C.I.

DANVAS

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PROJECT
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DRAWN
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PROJECT No:
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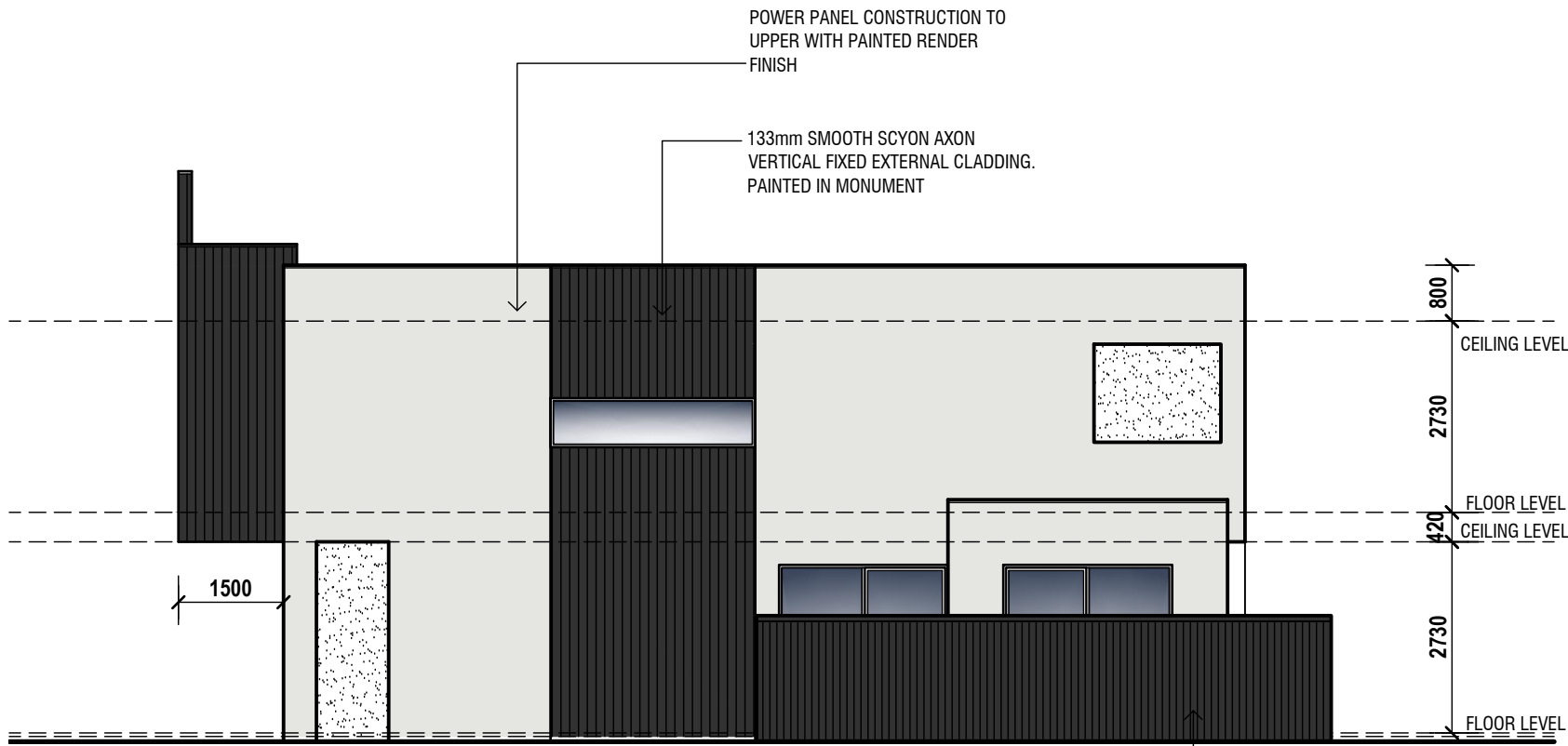
DRIVEWAY ELEVATION (WESTERN)

SCALE 1:100 @ A2



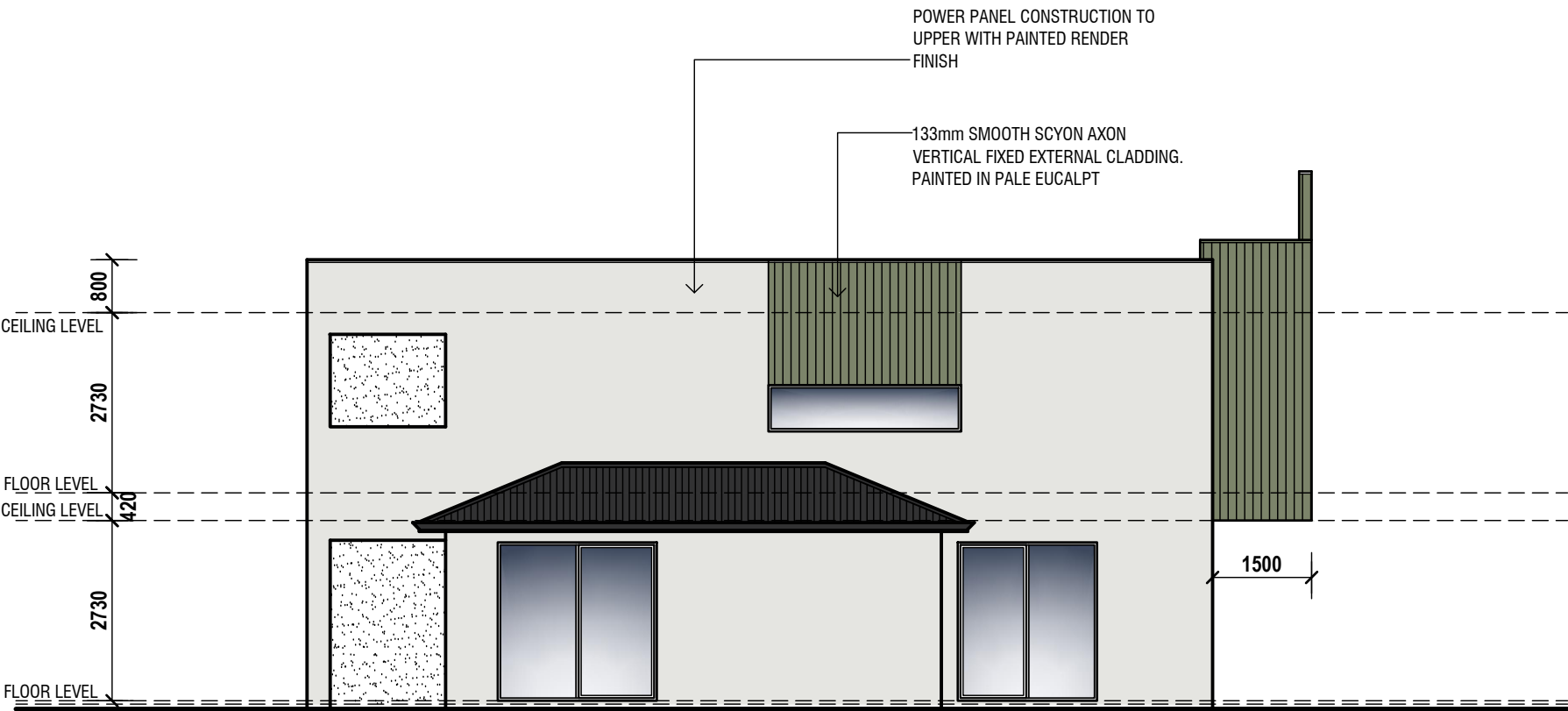
EASTERN ELEVATION

SCALE 1:100 @ A2



ELEVATION TO ALEXANDRINA ROAD (SOUTHERN)

SCALE 1:100 @ A2



NORTHERN ELEVATION

SCALE 1:100 @ A2

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Rev	Date	Description	Drawn
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C	29.07.25	Council RFI	C.I.

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PROJECT
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25.2025

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PA 07 OF 07

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1. This is an engineering survey only and should not be used as a boundary identification survey.
2. Owner to peg site prior to construction
3. All surplus soil to be removed by owner.
4. Stormwater, retaining walls & perimeter paving by owner. Stormwater and perimeter paving to be fully installed within 6 months of handover.
5. Cover to stormwater pipes in areas of vehicular traffic to be 300mm elsewhere 150mm.
6. ~~This site is classified as Class Flexible connections are necessary for all stormwater and sewer pipes.~~
7. For construction at or near boundary care must be taken to ensure stability of adjacent structures. Refer to Engineer if in doubt.
8. All stormwater disposal to satisfy Council's requirements.
9. The sewer run shown on this plan is indicative only. It is shown to determine whether sewer piers are required. Underfloor plumbing to be constructed as per Australian standards. Falls to be checked by builder prior to construction.

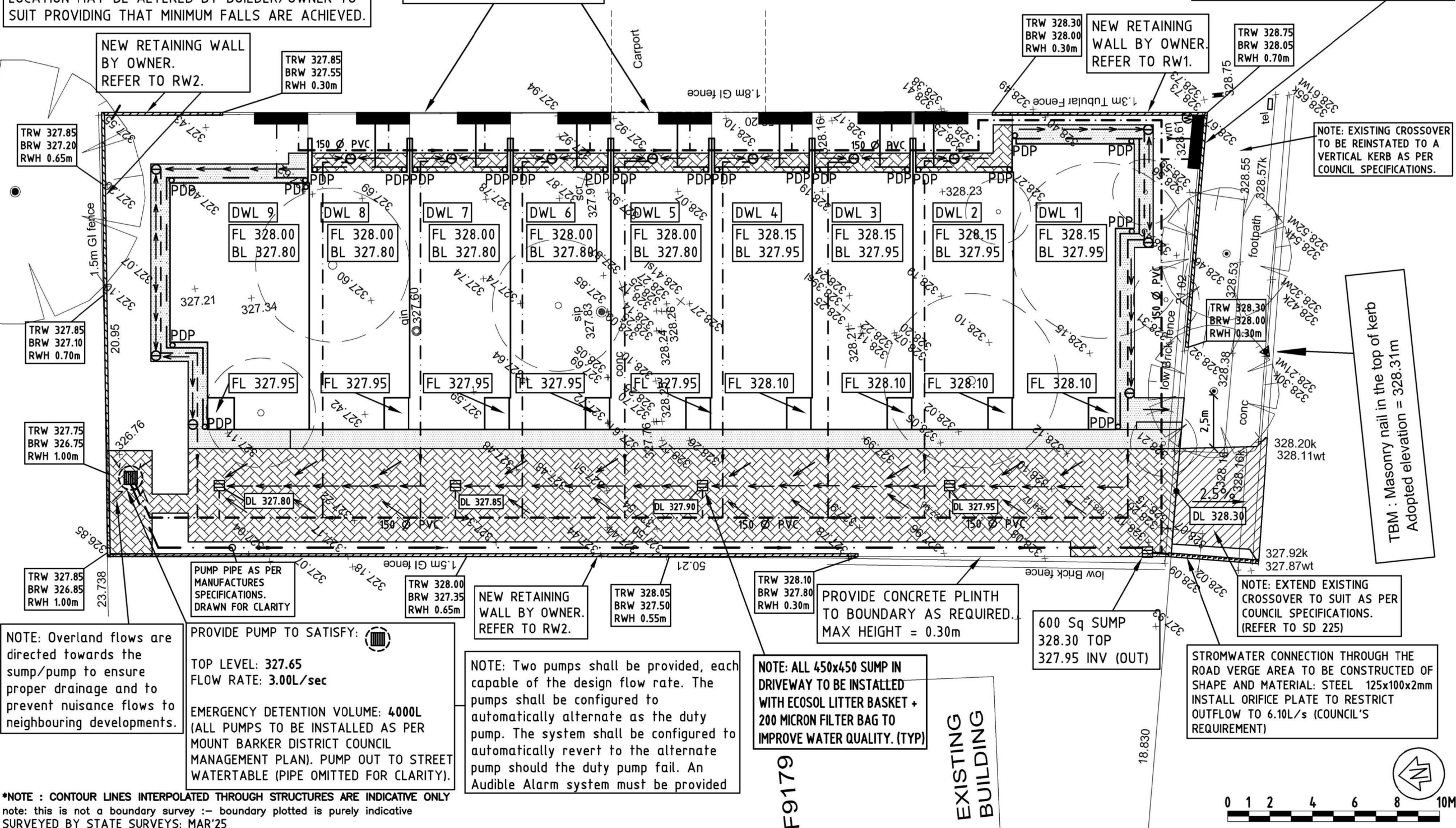
NOTE: ROOF DOWNPIPES MARKED 'PDP' TO BE DISCHARGED TO 4,000L RAINWATER/FRWT TANK VIA A SEALED SYSTEM (REFER TO 'SDS'). ALLOW TANK OVERFLOW TO STREET. ALL SURFACE STORMWATER TO BE DISCHARGED DIRECTLY TO SUMP/PUMP VIA A GRAVITY SYSTEM.

paving - refer PV1 Permeable paving x99.05 existing spot levels existing contours design batter 1 : 2 (u.n.o.) retaining wall as specified 100mm wide grated trench spoon drain & direction of flow Ø90 upvc stormwater pipe, min fall 1:200 Ø90 upvc stormwater pipe (Sealed system) sewer pipe, min fall 1:60	Borehole WM Water meter Sewer IP Telstra ETSA Block pegs FL design finished floor level BL design bench level WT design watertable level DL design ground / paving level TRW design top of retaining wall level RWH design retaining wall height DP ○ selected downpipe PDO ○ PVC downpipe 450 x 450 grated sump (u.n.o.) ⊖ Grated inlet 150 DIA
---	--

PROVIDE CONCRETE PLINTH
TO BOUNDARY AS REQUIRED.
MAX HEIGHT = 0.30m

NEW RETAINING WALL
BY OWNER.
REFER TO RW2.

TRW	327.85
BRW	327.55
RWH	0.30m



NOTE: Overland flows are directed towards the sump/pump to ensure proper drainage and to prevent nuisance flows to neighbouring developments.

PROVIDE PUMP TO SATISFY:

TOP LEVEL: 327.65
FLOW RATE: 3.00L/sec

EMERGENCY DETENTION VOLUME: 4000L
(ALL PUMPS TO BE INSTALLED AS PER
MOUNT BARKER DISTRICT COUNCIL
MANAGEMENT PLAN). PUMP OUT TO STREET
WATERTABLE (PIPE OMITTED FOR CLARITY).

NOTE: Two pumps shall be provided, each capable of the design flow rate. The pumps shall be configured to automatically alternate as the duty pump. The system shall be configured to automatically revert to the alternate pump should the duty pump fail. An Audible Alarm system must be provided

NOTE: ALL 450x450 SUMP IN DRIVEWAY TO BE INSTALLED WITH ECOSOL LITTER BASKET + 200 MICRON FILTER BAG TO IMPROVE WATER QUALITY. (TYP)

PROVIDE CONCRETE PLINTH
TO BOUNDARY AS REQUIRED.
MAX HEIGHT = 0.30m

600	Sq	SUMP
328.30	TOP	
327.95	INV	(OUT)

NOTE: EXTEND EXISTING CROSSOVER TO SUIT AS PER COUNCIL SPECIFICATIONS. (REFER TO SD 225)

STORMWATER CONNECTION THROUGH THE ROAD VERGE AREA TO BE CONSTRUCTED OF SHAPE AND MATERIAL: STEEL 125x100x2mm
INSTALL ORIFICE PLATE TO RESTRICT OUTFLOW TO 6.10L/s (COUNCIL'S REQUIREMENT)



TEK ENGINEERS
Consulting Engineers

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No.7 Joann Street
Athelstone, SA 5076
Telephone: 0403 193 096



MOUNT BARKER
DISTRICT COUNCIL

Planning Consent

SPECTRA

FOR: DANVAS DEVELOPMENTS
13 ALEXANDRINA ROAD
MOUNT BARKER

SITE & DRAINAGE PLAN

Revision :

'A': 18/07/25: PUMP/SUMP AMENDED (KB)

Drawn : SH

Design : KB

Date : MAY'25

Scale : 1:200

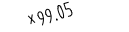
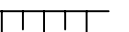
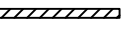
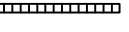
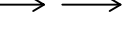
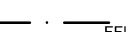
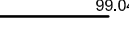
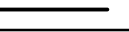
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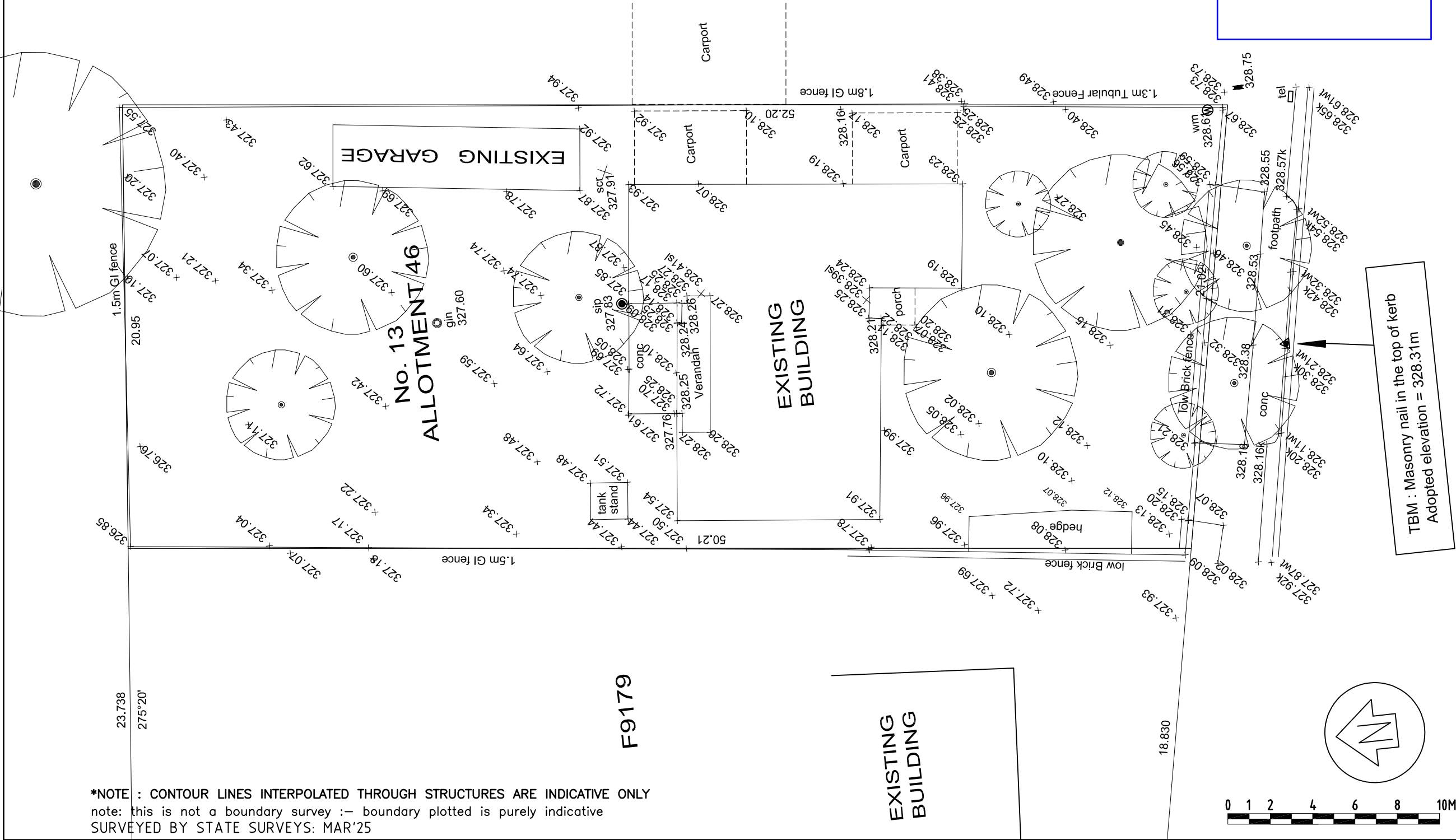
Sheet Number : SD1/A

NOTES

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- 5. Cover to stormwater pipes in areas of vehicular traffic to be 300mm elsewhere 150mm.
- 6. ~~This site is classified as Class Flexible connections are necessary for all stormwater and sewer pipes.~~
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- 8. All stormwater disposal to satisfy Council's requirements.
- 9. The sewer run shown on this plan is indicative only. It is shown to determine whether sewer piers are required. Underfloor plumbing to be constructed as per Australian standards. Falls to be checked by builder prior to construction.

LEGEND

	paving - refer SRP		Borehole		design finished floor level
	existing spot levels		Water meter		design bench level
	existing contours		Sewer IP		design watertable level
	design batter 1 : 2 (u.n.o.)		Telstra		design ground / paving level
	retaining wall as specified		ETSA		design top of retaining wall level
	100mm wide grated trench		Block pegs		design retaining wall height
	spoon drain & direction of flow	DP  selected downpipe			PDP  PVC downpipe
	FFL	 300 x 300 grated sump (u.n.o)			 Grated inlet 150 DIA
	Ø 90 upvc stormwater pipe, min fall 1:200				
	Ø 90 upvc stormwater pipe (Sealed system)				
	sewer pipe, min fall 1:60				



*NOTE : CONTOUR LINES INTERPOLATED THROUGH STRUCTURES ARE INDICATIVE ONLY
note: this is not a boundary survey :- boundary plotted is purely indicative
SURVEYED BY STATE SURVEYS: MAR'25



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SPECTRA

FOR: DANVAS DEVELOPMENTS
13 ALEXANDRINA ROAD
MOUNT BARKER

ENGINEERING SURVEY

Revision :	
Drawn : SH	
Design : SH	
Date : MAY'25	
Scale : 1:200	
Attachment : 17810	
Sheet Number :	ES



TEK ENGINEERS
Consulting Engineers

Job: 17810
Design: K.B.
Date: July'25
Page: SW1

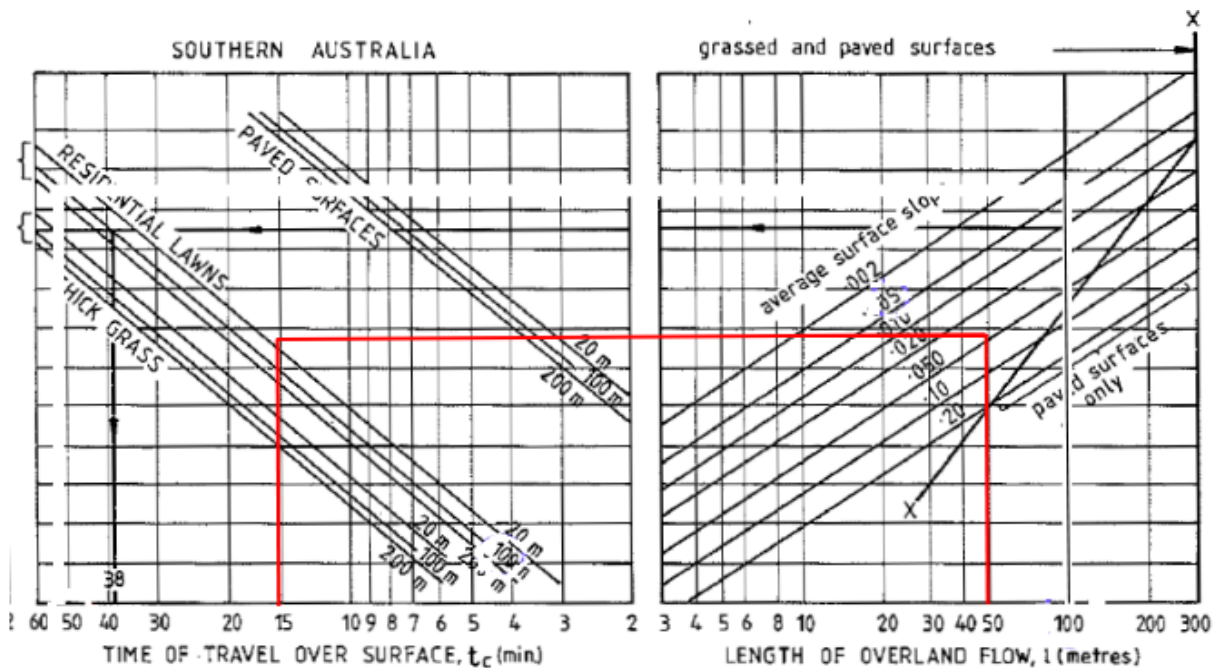


Planning Consent

TIME OF CONCENTRATION

$$\text{Time of concentration: } t_c = \frac{6.94 \times (L \times n)^{0.6}}{I^{0.4} \times S^{0.3}}$$

Design AEP: 10%
Distance (L): 50 m (house, short grass)
Mannings (n): 0.025 (vacant, grass)
Slope (S): 0.036 m/m (drop 1.8m over 50m)



Choose T_c that best match Duration,
Critical Time of Concentration =

15 mins



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Job: 17810
Design: K.B.
Date: July'25
Sheet: SW2

PRE-DEV FLOW

Site area = 10% AEP (10yr ARI)
1073 m²
Run-off coefficient (C) = 0.35
Time of concentration (t_c) = 15 mins
Rainfall intensity (I) = 58.6 mm/hr
Q_{pre} = CIA / 3600 = 6.1 l/s



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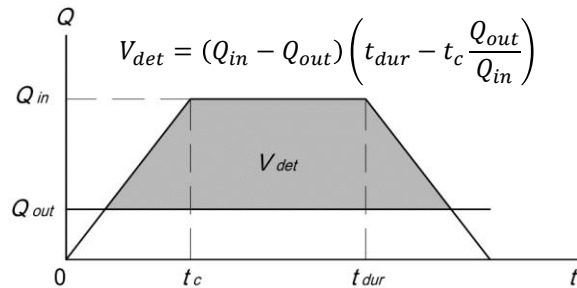
Job: 17810
Design: K.B.
Date: July'25
Sheet: SW3

POST-DEV FLOW

Design AEP: 10% (10yr ARI)
Roof = 540 m² C = 0.90
Paving = 84 m² C = 0.75
Lawn/Permeable paving = 449 m² C = 0.12
Time of concentration (t_c) = 15 mins

Select Q_{out} = 6.1 l/s (restricted to 10% AEP)

t _{dur} mins	I mm/hr	Q _{in} l/s	V _{det} cbm
5	99.8	16.71	-
10	72.7	12.17	-
15	58.6	9.81	1.3
20	49.7	8.32	1.2
25	43.5	7.28	0.9
30	38.9	6.51	0.4
35	35.4	5.93	-
40	32.5	5.44	-
45	30.1	5.04	-
50	28.1	4.71	-
55	26.4	4.42	-
60	25.0	4.19	-
90	19.1	3.20	-
120	15.7	2.63	-
180	11.9	1.99	-
240	9.7	1.63	-



	A	C	AxC	Coverage	Vdet
Roof	540	0.90	486	81%	1.0
Paving	84	0.75	63	10%	0.1
Lawn	449	0.12	53.88	9%	0.1
	1073	0.56	602.88	100%	1.3

(see page SW5 -->)



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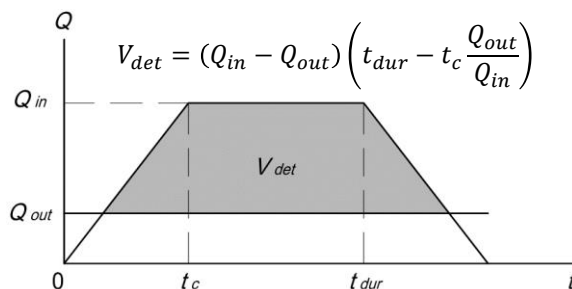
Job: 17780
Design: K.B.
Date: May'25
Sheet: SW4

POST-DEV FLOW

Design AEP: 1% (10yr ARI)
Roof = 540 m² C = 0.90
Paving = 84 m² C = 0.75
Lawn/Permeable paving = 449 m² C = 0.12
Time of concentration (t_c) = 15 mins

Select Q_{out} = 6.1 l/s (restricted to 10% AEP)

t _{dur} mins	I mm/hr	Q _{in} l/s	V _{det} cbm
5	174.0	29.14	-
10	126.0	21.10	-
15	102.0	17.08	6.3
20	86.4	14.47	6.8
25	75.8	12.69	7.0
30	67.8	11.35	6.9
35	61.6	10.32	6.6
40	56.6	9.48	6.1
45	52.5	8.79	5.6
50	49.1	8.22	4.9
55	46.1	7.72	4.2
60	43.5	7.28	3.3
90	33.1	5.54	-
120	27.1	4.54	-
180	20.3	3.40	-
240	16.5	2.76	-



	A	C	AxC	Coverage	Vdet
Roof	540	0.90	486	81%	5.7
Paving	84	0.75	63	10%	0.7
Lawn	449	0.12	53.88	9%	0.6
	1073	0.56	602.88	100%	7.0

(see page SW6 -->)



TEK ENGINEERS
Consulting Engineers

Job: 17810
Design: K.B.
Date: July'25
Sheet SW5

POST-DEV FLOW (CONT.)

DESIGN SUMMARY

DESIGN AEP: 10% (10yr ARI)

Required detention:

Roof:	1.0 cbm
Paving + lawn:	<u>0.2 cbm</u>
	1.3 cbm

Provided:

Roof:	9.0 cbm via above ground water tanks (1.0 cbm per dwelling)
Paving + lawn:	<u>4.0 cbm via sump/pump (underground)</u>
	13.0 cbm ::: satisfied

DESIGN AEP: 1%

Required detention:

Roof:	5.7 cbm
Paving + lawn:	<u>1.4 cbm</u>
	7.0 cbm

Provided:

Roof:	9.0 cbm via above ground water tanks (1.0 cbm per dwelling)
Paving + lawn:	<u>4.0 cbm via sump/pump (underground)</u>
	13.0 cbm ::: satisfied

PUMP & SUMP DESIGN CALCULATIONS



Date: 18-Jul-25
Page: SW6/A

TEK ENGINEERS
Consulting Engineers

Job:	17810
Site:	13 ALEXANDRINA ROAD MOUNT BARKER
For:	DANVAS DEVELOPMENTS

1. SUMP AND PUMP CALCULATIONS

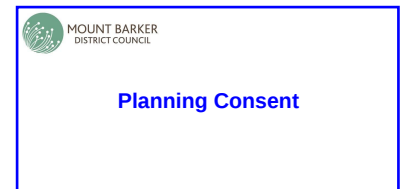
1.1 STORAGE VOLUME FOR 1:100 STORM EVENT WITH A SELECTED PUMPING RATE

Design storm

1 : 100 yr

Eaves gutter storage capacity **1 : 20 yr** (to calculate gap flow to 1 in 100 yr)

New roof area	0	0.90
Paved area to pump	84	0.75
Garden/permeable paving area to pump	449	0.12
Total area	533	0.22



Try pump rate **3.0** l/sec

DUR.	1 : 100 yr ARI	1 : 20 yr ARI	1 : 100 yr Q	1 : 20 yr Q	VOL. IN	VOL. OUT	DET. VOL.
5 mins	186.00	121.00	6.04	0.00	1812	900	912
7 mins	161.00	105.00	5.23	0.00	2195	1260	935
10 mins	136.00	89.00	4.42	0.00	2649	1800	849
15 mins	110.00	72.00	3.57	0.00	3214	2700	514
20 mins	94.00	62.00	3.05	0.00	3662	3600	62
25 mins	82.00	54.00	2.66	0.00	3993	4500	-507
30 mins	73.40	48.70	2.38	0.00	4289	5400	-1111
35 mins	66.60	44.30	2.16	0.00	4541	6300	-1759
40 mins	61.00	40.70	1.98	0.00	4753	7200	-2447
45 mins	57.00	37.80	1.85	0.00	4997	8100	-3103
50 mins	53.00	35.30	1.72	0.00	5162	9000	-3838
55 mins	49.50	33.20	1.61	0.00	5303	9900	-4597
1 hr	46.60	31.30	1.51	0.00	5447	10800	-5353
2 hrs	29.10	19.80	0.94	0.00	6802	21600	-14798

1.2. STORAGE VOLUME FOR PUMP FAILURE

Detention volume is based on **1 : 10 yr** **2 hrs** storm as per councils requirement.

ARI = 16.4 mm/hr

$$\text{Required volume} = \frac{CIA}{3600} \times t = 3834 \text{ L}$$

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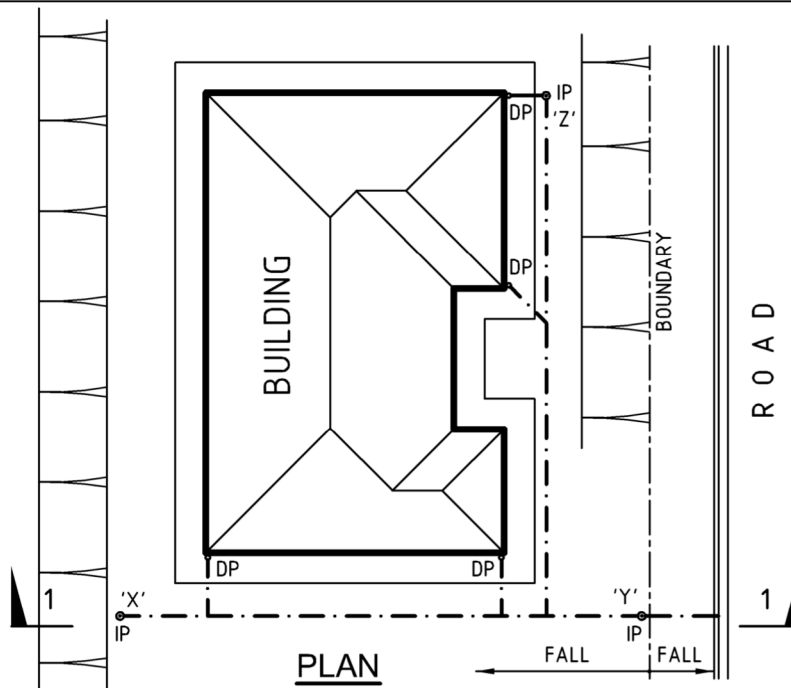
SEALED STORMWATER AND EAVES GUTTER SYSTEM

Drawn : BA

Design : BA

Date : MAY'23

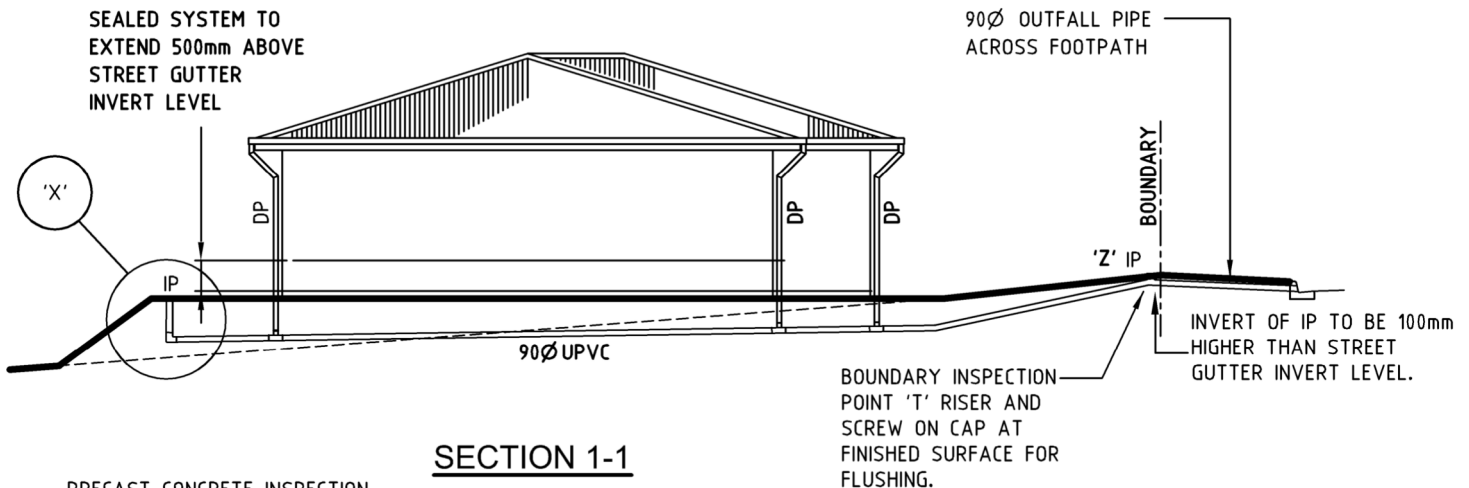
Sheet Number : SSD1



NOTES:

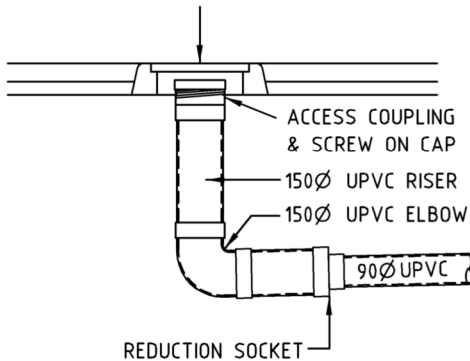
1. SEALED SYSTEM TO BE CONSTRUCTED TO PRESSURE LINE STANDARD (GLUED JOINTS FOR UPVC) STATIC TESTED BEFORE BACKFILLING.
2. NO SURFACE INLETS (SUMP, GRATED PITS ETC.) PERMITTED IN SEALED SYSTEM.
3. DO NOT USE SEAMED METAL DOWN PIPES OR FITTINGS.
4. 'Z' DENOTES RISER AND SCREW ON INSPECTION CAP AT FINISHED SURFACE LEVEL OR CLEANING EYE IN DP FOR RODDING OR FLUSHING PURPOSES AT 'DEAD END' BRANCHES.
5. REGULAR FLUSHING AND MAINTENANCE BY OWNER IS REQUIRED.

SEALED SYSTEM TO
EXTEND 500mm ABOVE
STREET GUTTER
INVERT LEVEL



SECTION 1-1

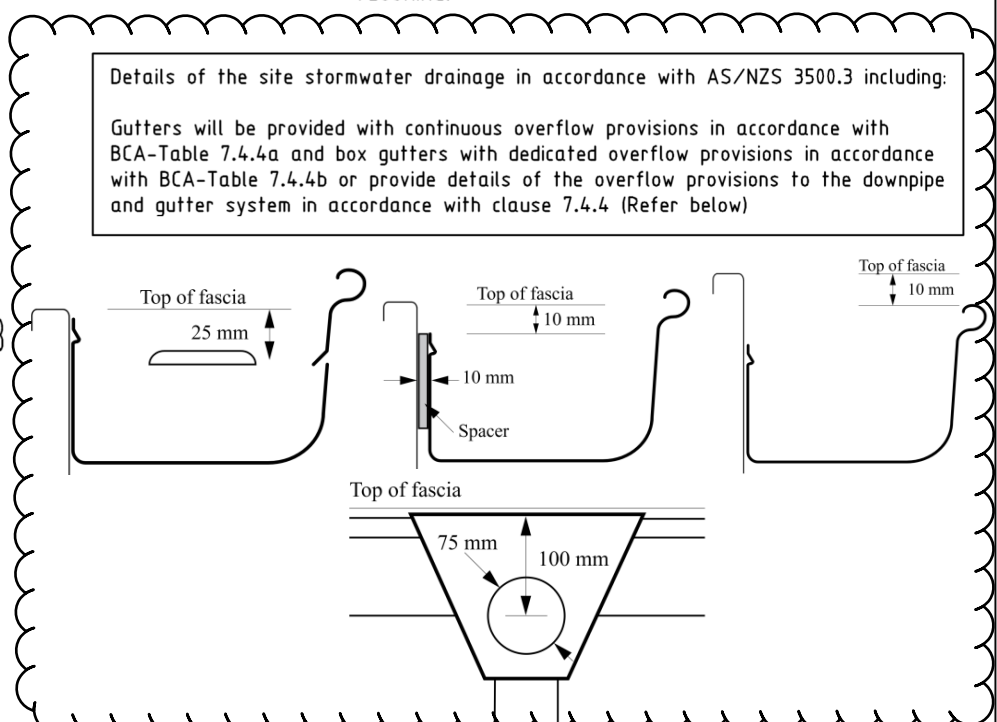
PRECAST CONCRETE INSPECTION
POINT COVER AND SURROUND
SET IN AT PAVING LEVEL



DETAIL X

Details of the site stormwater drainage in accordance with AS/NZS 3500.3 including:

Gutters will be provided with continuous overflow provisions in accordance with BCA-Table 7.4.4a and box gutters with dedicated overflow provisions in accordance with BCA-Table 7.4.4b or provide details of the overflow provisions to the downpipe and gutter system in accordance with clause 7.4.4 (Refer below)



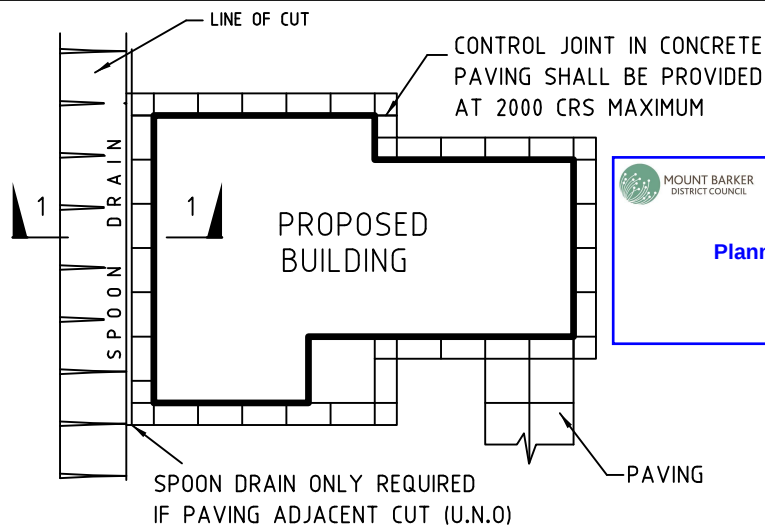
STANDARD PAVING DETAILS

Drawn : BA

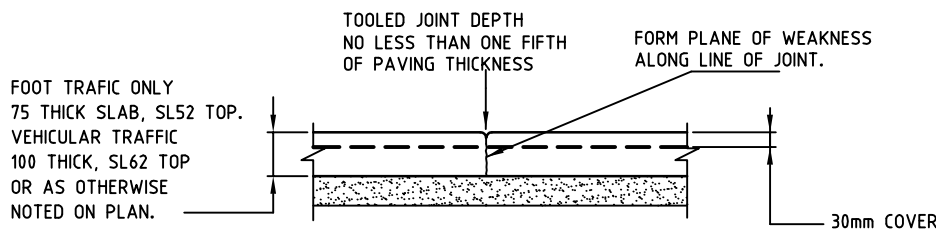
Design : BA

Date : OCT'21

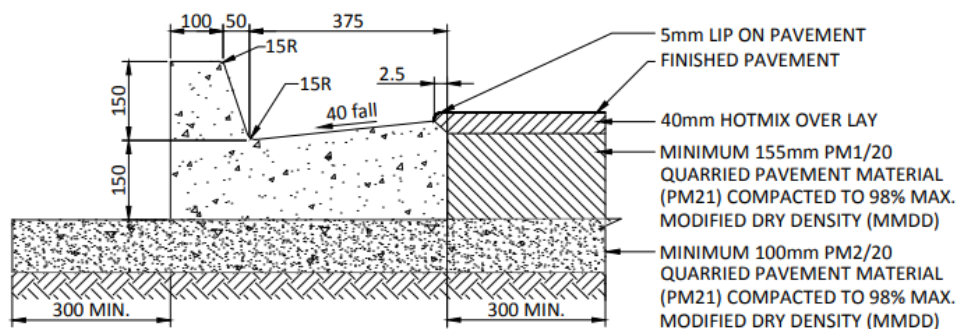
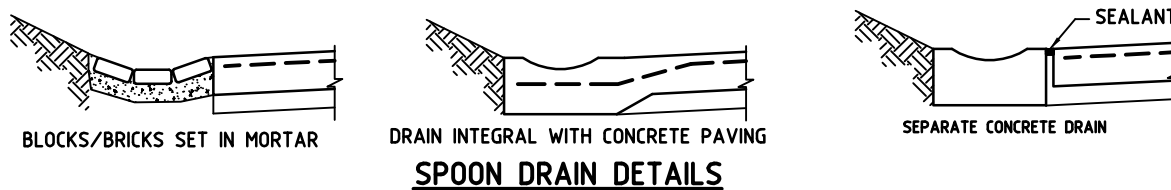
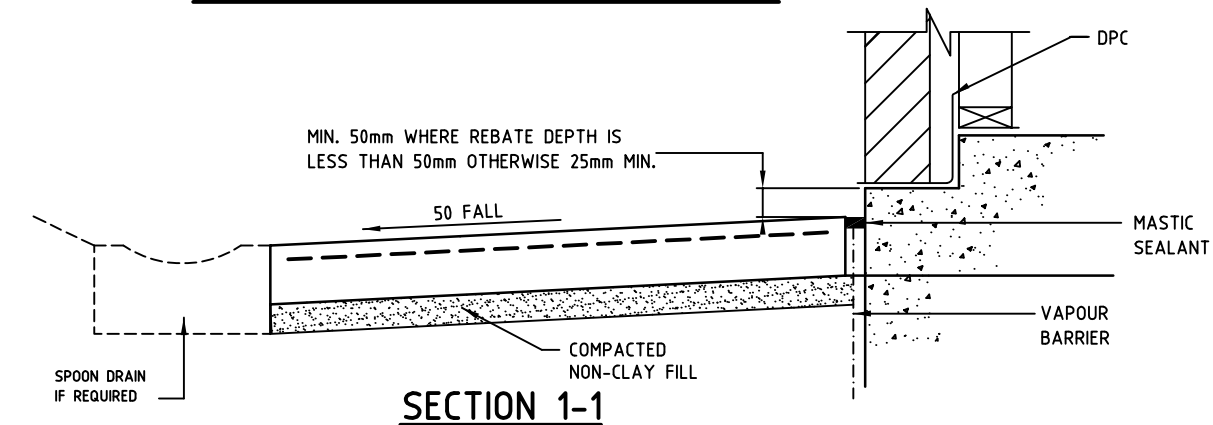
Sheet Number : PV1



PLAN OF PAVING AROUND BUILDING



CONTROL JOINT DETAIL IN PAVING



150mm KERB AND GUTTER

NOTES

1. PAVING MUST NOT BRIDGE THE DAMP PROOF COURSE (DPC).
2. PAVING SHALL BE GRADED TO ENSURE ALL WATER CAN DRAIN AWAY FROM THE BUILDING.
3. PROVIDE EDGE DRAINS WHERE NECESSARY TO DIVERT RUNOFF.
4. PAVING IS A NON-STRUCTURAL COMPONENT. ON REACTIVE SITES H-D, E-D THE PAVING MAY REQUIRE RE-LAYING AT TIMES DUE TO HIGH SOIL MOVEMENTS.

1. CONNECTORS ARE REQUIRED, AS DETAILED ABOVE, AT EACH CONNECTION.
2. CONNECTIONS ARE REQUIRED AT EACH LOCATION WHERE ANY PIPES (60mm OR GREATER) PENETRATE OR PASS BENEATH THE EXTERNAL FOOTING BEAMS.
3. THE DENSO TAPE MUST BE SPRINKLED WITH COPPER SULPHATE CRYSTALS PRIOR TO WRAPPING AND MUST EXTEND 50mm BEYOND THE JOINT ON EITHER SIDE.
4. DETAILS ARE APPLICABLE TO SEWER AND STORMWATER PIPES.

DETENTION TANK SERIES (CONCEPT ONLY)

NOTES

1. TANKS MAY BE CONNECTED BY Ø90mm UNDERGROUND SYSTEM PROVIDED INSPECTION OPENINGS ARE LOCATED AT ALL VERTICAL & HORIZONTAL BENDS.
2. WHERE TANKS ARE CONNECTED IN SERIES A 'FULL DETENTION TANK' MUST END THE SERIES.
3. A DETENTION TANK SERIES MAY HAVE ONLY ONE OUTLET FROM THE PROPERTY & THE OUTLET MUST BE LOCATED ON THE LAST TANK OF THE SERIES.

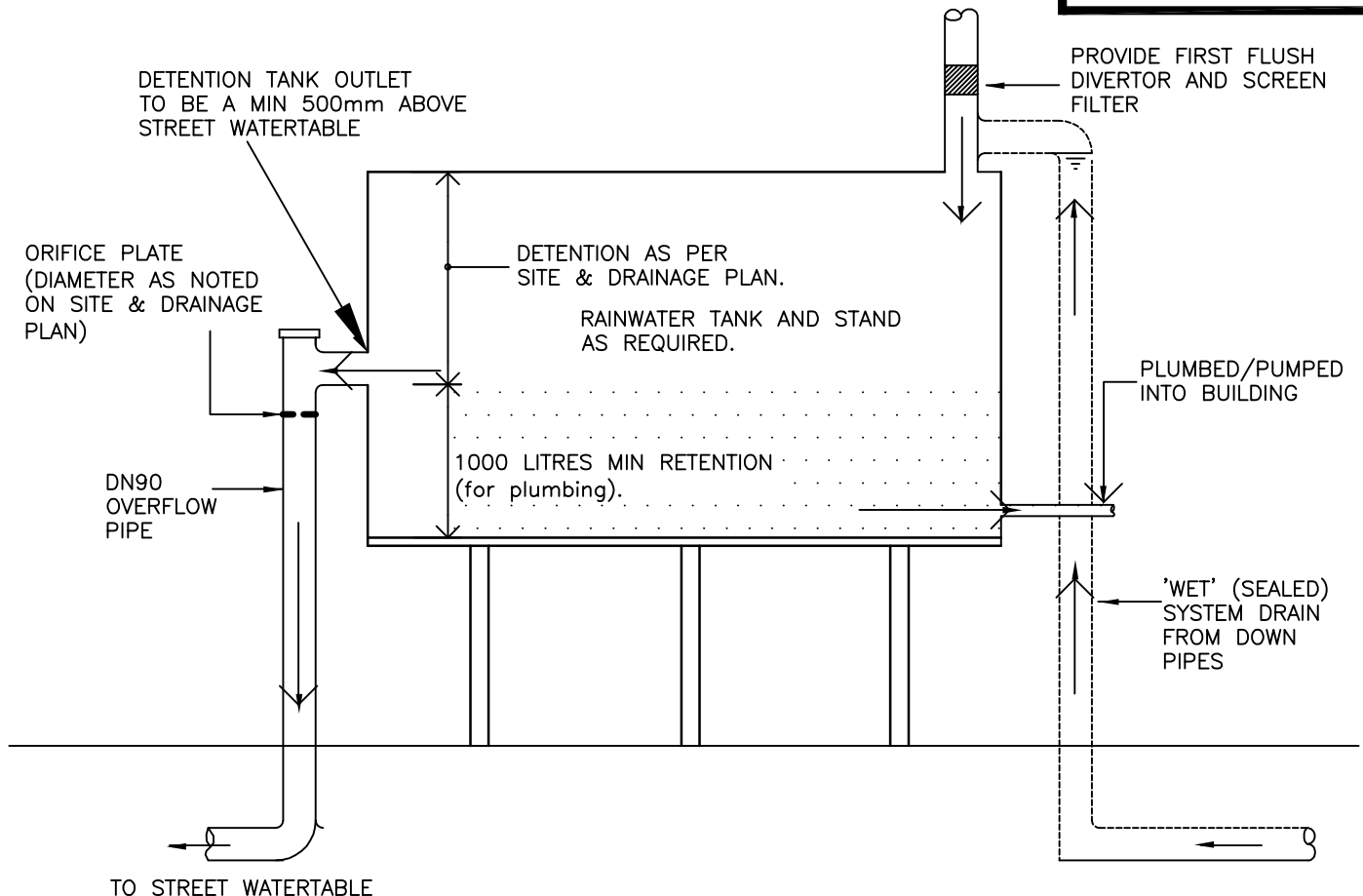
DETENTION TANK DETAIL

Drawn : BA

Design : BA

Date : MAY'17

Sheet Number : DT



DETENTION / STORAGE

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SLEEPER RETAINING WALL (RETAINING CUT)

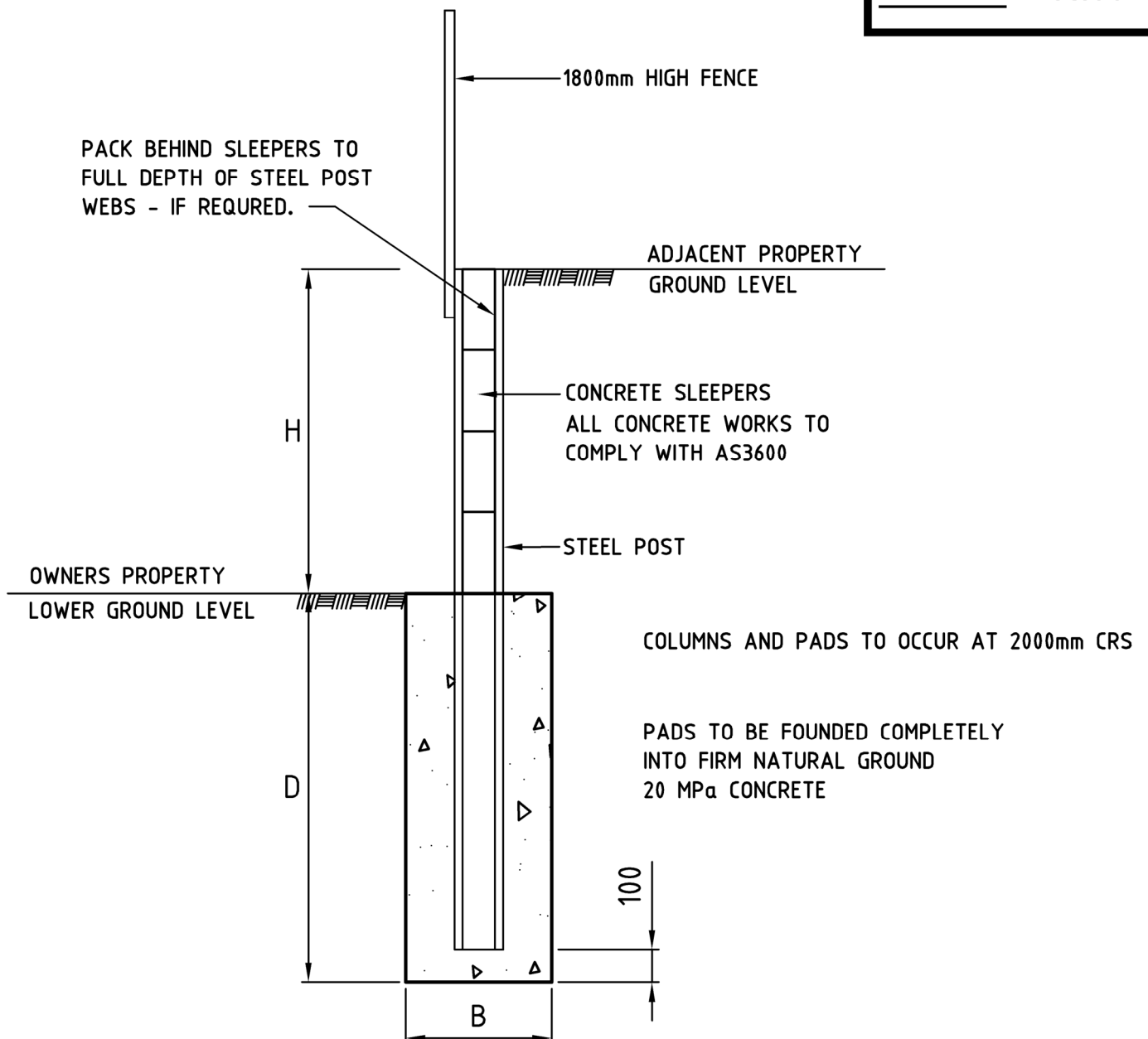
Drawn : BA

Design : BA

Date : MAR'25

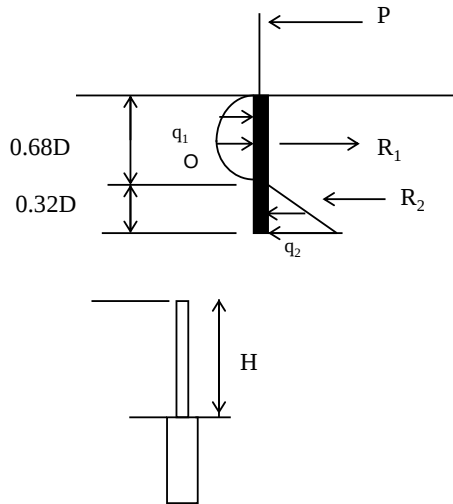
Sheet Number : RW1

WALL HEIGHT (H) mm	PIER DEPTH (D) mm	PIER DIAMETER (B) mm	STEEL POST
600	800	450	150UB14
1000	1200	450	150UB14
1200	1400	450	150UB14
1500	1700	600	150UB14
1800	1900	600	180UB18
2100	2200	600	200UB22
2400	2600	600	200UC46
2700	2400	900	200UC46
3000	2800	900	200UC46



SLEEPER RETAINING WALL DESIGN (RETAINING CUT)

RETAINING WALL ANALYSIS

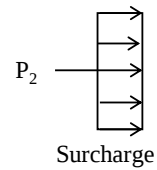
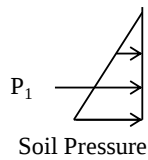


$$R_1 = q_1(2/3)(0.68DB)$$

$$R_2 = q_2(0.5)(0.32DB)$$

$$R_1 = P + R_2$$

$$\text{Sum } M_o = 0$$



SOIL PROPERTIES

$K_a =$	0.4	
Surcharge =	5	kPa
Unit Weight =	17	kN/m ³
Passive Pressure, $q_2 =$	100	kPa

WALL HEIGHT

0.60 m

Pier Spacing

2.00 m

Passive Pressure, $q_2 =$

100 kPa

$q_1 =$

47.15 kPa

Footing Pier

$P_1 =$	1.22	kN/m
$P_2 =$	1.20	kN/m
Diameter =	0.45	m
$M_o =$	0.0	kNm
Depth =	0.50	m

USE 450 mm x 600 mm deep piers @ 2000 mm centres

Steel Posts

$$Z_{\text{req}} = 7.3\text{E}+3 \text{ mm}^3$$

USE 150UB14 posts @ 2000 mm centres



WALL HEIGHT 1.00 m
Pier Spacing 2.00 m
Passive Pressure, $q_2 =$ 100 kPa
 $q_1 =$ 48.53 kPa

Footing Pier
 $P_1 =$ 3.40 kN/m
 $P_2 =$ 2.00 kN/m
Diameter = 0.45 m
 $M_o =$ 0.0 kNm
Depth = 1.00 m

USE 450 mm x 1100 mm deep piers @ 2000 mm centres

Steel Posts

$$Z_{\text{req}} = 25.9\text{E}+3 \text{ mm}^3$$

USE 150UB14 posts @ 2000 mm centres

WALL HEIGHT 1.20 m
Pier Spacing 2.00 m
Passive Pressure, $q_2 =$ 100 kPa
 $q_1 =$ 49.89 kPa

Footing Pier
 $P_1 =$ 4.90 kN/m
 $P_2 =$ 2.40 kN/m
Diameter = 0.45 m
 $M_o =$ 0.0 kNm
Depth = 1.23 m

USE 450 mm x 1300 mm deep piers @ 2000 mm centres

Steel Posts

$$Z_{\text{req}} = 41.2\text{E}+3 \text{ mm}^3$$

USE 150UB14 posts @ 2000 mm centres

WALL HEIGHT 1.50 m
Pier Spacing 2.00 m
Passive Pressure, $q_2 =$ 100 kPa
 $q_1 =$ 48.61 kPa

Footing Pier
 $P_1 =$ 7.65 kN/m
 $P_2 =$ 3.00 kN/m
Diameter = 0.6 m
 $M_o =$ 0.0 kNm
Depth = 1.47 m

USE 600 mm x 1600 mm deep piers @ 2000 mm centres

Steel Posts

$$Z_{\text{req}} = 73.6\text{E}+3 \text{ mm}^3$$

USE 150UB14 posts @ 2000 mm centres



WALL HEIGHT **1.80 m**
Pier Spacing **2.00 m**
 Passive Pressure $q_2 =$ **150.0 kPa**
 $q_1 =$ **70.85 kPa**

Footing Pier

$P_1 =$ **11.02 kN/m**
 $P_2 =$ **3.60 kN/m**
 Diameter = **0.6 m**
 $M_o =$ **0.00 kNm**
 Depth = **1.50 m**

USE 600 mm x 1700 mm deep piers @ 2000 mm centres

Steel Posts

$$Z_{\text{req}} = 119.4\text{E}+3 \text{ mm}^3$$

USE 180UB18 posts @ 2000 mm centres

WALL HEIGHT **2.10 m**
Pier Spacing **2.00 m**
 Passive Pressure, $q_2 =$ **150.0 kPa**
 $q_1 =$ **72.06 kPa**

Footing Pier

$P_1 =$ **14.99 kN/m**
 $P_2 =$ **4.20 kN/m**
 Diameter = **0.6 m**
 $M_o =$ **0.0 kNm**
 Depth = **1.85 m**

USE 600 mm x 2000 mm deep piers @ 2000 mm centres

Steel Posts

$$Z_{\text{req}} = 180.7\text{E}+3 \text{ mm}^3$$

USE 200UB22 posts @ 2000 mm centres

WALL HEIGHT **2.40 m**
Pier Spacing **2.00 m**
 Passive Pressure, $q_2 =$ **150 kPa**
 $q_1 =$ **72.86 kPa**

Footing Pier

$P_1 =$ **19.58 kN/m**
 $P_2 =$ **4.80 kN/m**
 Diameter = **0.6 m**
 $M_o =$ **0.0 kNm**
 Depth = **2.25 m**

USE 600 mm x 2400 mm deep piers @ 2000 mm centres

Steel Posts

$$Z_{\text{req}} = 259.7\text{E}+3 \text{ mm}^3$$

USE 200UC46 posts @ 2000 mm centres

WALL HEIGHT 2.70 m
Pier Spacing 2.00 m
Passive Pressure, $q_2 =$ 150 kPa
 $q_1 =$ 70.82 kPa



Footing Pier
 $P_1 =$ 24.79 kN/m
 $P_2 =$ 5.40 kN/m
Diameter = 0.9 m
 $M_o =$ 0.0 kNm
Depth = 2.07 m

USE 900 mm x 2200 mm deep piers @ 2000 mm centres

Steel Posts

$$Z_{\text{req}} = 358.8\text{E}+3 \text{ mm}^3$$

USE 200UC46 posts @ 2000 mm centres

WALL HEIGHT 3.00 m
Pier Spacing 2.00 m
Passive Pressure, $q_2 =$ 150 kPa
 $q_1 =$ 71.45 kPa

Footing Pier
 $P_1 =$ 30.60 kN/m
 $P_2 =$ 6.00 kN/m
Diameter = 0.9 m
 $M_o =$ 0.0 kNm
Depth = 2.42 m

USE 900 mm x 2600 mm deep piers @ 2000 mm centres

Steel Posts

$$Z_{\text{req}} = 400.0\text{E}+3 \text{ mm}^3$$

USE 200UC46 posts @ 2000 mm centres

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SLEEPER RETAINING WALL (RETAINING FILL)

Drawn : BA

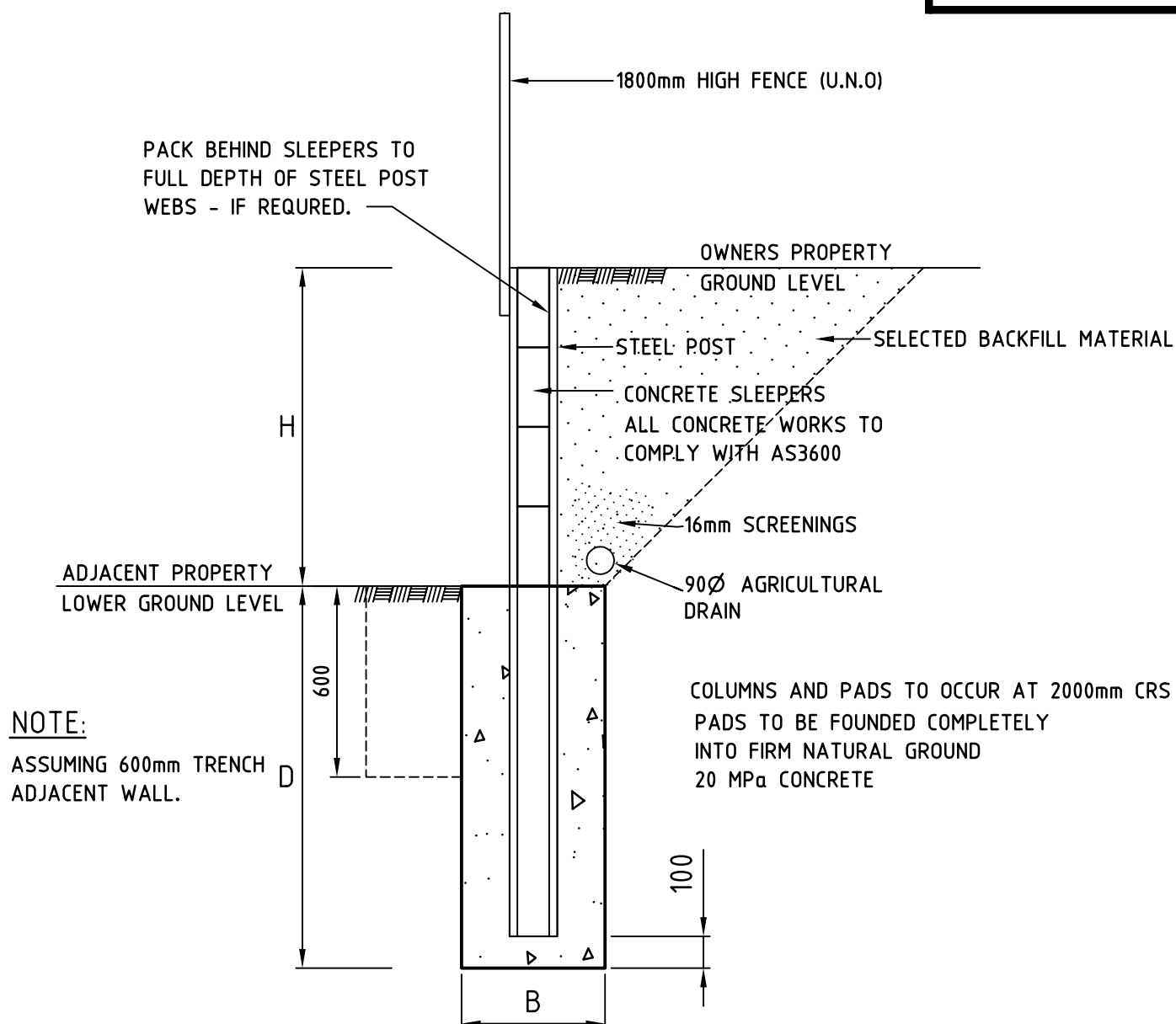
Design : BA

Date : APR'21

Sheet Number :

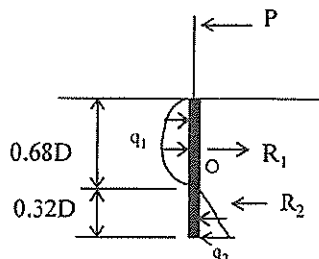
RW2

WALL HEIGHT (H) mm	PIER DEPTH (D) mm	PIER DIAMETER (B) mm	STEEL POST
600	1300	600	150UB14
1000	1600	600	150UB18
1200	1800	600	180UB18
1500	2100	600	200UB18
1800	2500	600	200UB25
2100	2800	600	200UC46
2400	2700	900	200UC46
2700	3000	900	200UC52
3000	3300	900	250UC73



SLEEPER RETAINING WALL DESIGN (RETAINING FILL)

(Assuming 600 mm trench adjacent wall)

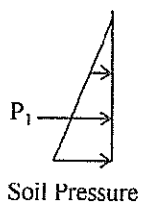
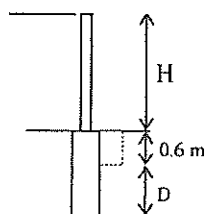
**RETAINING WALL ANALYSIS**

$$R_1 = q_1(2/3)(0.68DB)$$

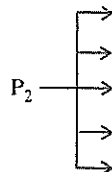
$$R_2 = q_2(0.5)(0.32DB)$$

$$R_1 = P + R_2$$

$$\text{Sum } M_o = 0$$



Soil Pressure



Surcharge

SOIL PROPERTIES

$$K_a = 0.3$$

$$\text{Surcharge} = 5 \text{ kPa}$$

$$\text{Unit Weight} = 17 \text{ kN/m}^3$$

$$\text{Passive Pressure, } q_2 = 150 \text{ kPa}$$

$$\text{WALL HEIGHT} = 0.60 \text{ m}$$

$$\text{Pier Spacing} = 2.00 \text{ m}$$

$$\text{Passive Pressure, } q_2 = 150 \text{ kPa}$$

$$q_1 = 67.4 \text{ kPa}$$

Footing Pier

$$P_1 = 3.67 \text{ kN/m}$$

$$P_2 = 1.80 \text{ kN/m}$$

$$\text{Diameter} = 0.6 \text{ m}$$

$$M_o = 0.00 \text{ kNm}$$

$$\text{Depth} = 0.70 \text{ m}$$

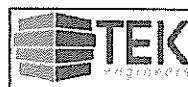
USE 600 mm x 1300 mm deep piers @ 2000 mm centres

Steel Posts

$$Z_{req} = 30.9E+3 \text{ mm}^3$$

USE 150UB14 posts @ 2000 mm centres

WALL HEIGHT 1.00 m
Pier Spacing 2.00 m
 Passive Pressure, $q_2 =$ 150 kPa
 $q_1 =$ 68.7 kPa



Footing Pier

$P_1 =$ 6.53 kN/m
 $P_2 =$ 2.40 kN/m
 Diameter = 0.6 m
 $M_o =$ 0.00 kNm
 Depth = 1.04 m

USE 600 mm x 1600 mm deep piers @ 2000 mm centres

Steel Posts

$$Z_{req} = 65.5E+3 \text{ mm}^3$$

USE 150UB18 posts @ 2000 mm centres

WALL HEIGHT 1.20 m
Pier Spacing 2.00 m
 Passive Pressure, $q_2 =$ 150 kPa
 $q_1 =$ 69.3 kPa

Footing Pier

$P_1 =$ 8.26 kN/m
 $P_2 =$ 2.70 kN/m
 Diameter = 0.6 m
 $M_o =$ 0.00 kNm
 Depth = 1.23 m

USE 600 mm x 1800 mm deep piers @ 2000 mm centres

Steel Posts

$$Z_{req} = 89.5E+3 \text{ mm}^3$$

USE 180UB18 posts @ 2000 mm centres

WALL HEIGHT 1.50 m
Pier Spacing 2.00 m
 Passive Pressure, $q_2 =$ 150 kPa
 $q_1 =$ 70.2 kPa

Footing Pier

$P_1 =$ 11.25 kN/m
 $P_2 =$ 3.15 kN/m
 Diameter = 0.6 m
 $M_o =$ 0.00 kNm
 Depth = 1.54 m

USE 600 mm x 2100 mm deep piers @ 2000 mm centres

Steel Posts

$$Z_{req} = 135.5E+3 \text{ mm}^3$$

USE 200UB18 posts @ 2000 mm centres



WALL HEIGHT 1.80 m
Pier Spacing 2.00 m
 Passive Pressure, $q_2 = 150$ kPa
 $q_1 = 70.9$ kPa

Footing Pier

$P_1 = 14.69$ kN/m
 $P_2 = 3.60$ kN/m
 Diameter = 0.6 m
 $M_o = 0.00$ kNm
 Depth = 1.87 m

USE 600 mm x 2500 mm deep piers @ 2000 mm centres

Steel Posts

$$Z_{req} = 194.8E+3 \text{ mm}^3$$

USE 200UB25 posts @ 2000 mm centres

WALL HEIGHT 2.10 m
Pier Spacing 2.00 m
 Passive Pressure, $q_2 = 150$ kPa
 $q_1 = 71.6$ kPa

Footing Pier

$P_1 = 18.59$ kN/m
 $P_2 = 4.05$ kN/m
 Diameter = 0.6 m
 $M_o = 0.00$ kNm
 Depth = 2.23 m

USE 600 mm x 2800 mm deep piers @ 2000 mm centres

Steel Posts

$$Z_{req} = 269.1E+3 \text{ mm}^3$$

USE 200UC46 posts @ 2000 mm centres

WALL HEIGHT 2.40 m
Pier Spacing 2.00 m
 Passive Pressure, $q_2 = 150$ kPa
 $q_1 = 69.6$ kPa

Footing Pier

$P_1 = 22.95$ kN/m
 $P_2 = 4.50$ kN/m
 Diameter = 0.9 m
 $M_o = 0.00$ kNm
 Depth = 2.02 m

USE 900 mm x 2700 mm deep piers @ 2000 mm centres

Steel Posts

$$Z_{req} = 360.0E+3 \text{ mm}^3$$

USE 200UC46 posts @ 2000 mm centres



WALL HEIGHT 2.70 m
Pier Spacing 2.00 m
 Passive Pressure, $q_2 =$ 150 kPa
 $q_1 =$ 70.1 kPa

Footing Pier

$P_1 =$ 27.77 kN/m
 $P_2 =$ 4.95 kN/m
 Diameter = 0.9 m
 $M_o =$ 0.00 kNm
 Depth = 2.33 m

USE 900 mm x 3000 mm deep piers @ 2000 mm centres

Steel Posts

$$Z_{req} = 469.3E+3 \text{ mm}^3$$

USE 200UC52 posts @ 2000 mm centres

WALL HEIGHT 3.00 m
Pier Spacing 2.00 m
 Passive Pressure, $q_2 =$ 150 kPa
 $q_1 =$ 70.6 kPa

Footing Pier

$P_1 =$ 33.05 kN/m
 $P_2 =$ 5.40 kN/m
 Diameter = 0.9 m
 $M_o =$ 0.00 kNm
 Depth = 2.66 m

USE 900 mm x 3300 mm deep piers @ 2000 mm centres

Steel Posts

$$Z_{req} = 598.5E+3 \text{ mm}^3$$

USE 250UC73 posts @ 2000 mm centres